Review Article

Analysis of Simply Supported Laminated Plate Using CLT and FSDT

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Abstract

Composite is the special kind of engineered materials. The mechanics of the composite are a little bit complicated. In the present work, the classical laminate theory (CLT) and first-order shear deformation theory (FSDT) is used for the finding the deformation/deflection in the simply supported composite plate loaded with the uniformly distributed load. The Navier solution applied to find the desired result.

Keywords: Composite, Lamina, Laminate, Stacking Sequence

1. Introduction

Composite materials are those that are made by combining two or more materials on a macroscopic scale in such a way that they have enhanced engineering qualities than the standard materials, such as metals. These materials are known as "composite materials." Stiffness, strength, resistance to corrosion, thermal characteristics, fatigue life, wear resistance [1-28], and effective decrease in weight are some of the attributes that may be enhanced. The vast majority of man-made composite materials are constructed using two different kinds of materials: a substance known as fibre for providing reinforcement and a material known as matrix material serving as the foundation or parent material. A typical sheet made of composite material is referred to as a lamina or ply. It is a vital component that makes up the whole. Numerous fibres are implanted in a matrix material to create a fiber-reinforced lamina [29-43]. This matrix material may be made of a metal, such as aluminium, or it can be made of a non-metal, such as thermoset or thermoplastic polymer. A laminate is a collection of lamina that have been layered in order to obtain the necessary stiffness and thickness. For example, lamina that are reinforced with unidirectional fibres may be stacked in such a way that the fibres in each lamina are orientated in the same or opposite directions. Both the laminates and the materials benefit from an improvement in their tribological characteristics as a result of the orientation [44-86]. Gears and other machine components often make use of the composite material. There have been a number of researches [87-159] conducted on the topic of gear failure as well as design and dependability. As a result, it is essential to do research on composites since they are a potential future material.

The orientation of the plies determines the two distinct kinds of laminates that may be created. [160]

A. Angle Ply

Angle-ply laminates have ply configurations of θ and - θ where $0^{\circ} \leq \theta \leq 90^{\circ}$, and at least one stack has an alignment other than 0° or 90°, as shown in Fig. 1.



Fig.1 Angle ply laminates

B. Cross Ply

Cross-ply laminates are those which have ply orientations of 0° or 90° as shown in Fig. 2



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2. Theories of analysis and their mathematical modelling

Calculating the deflection and stress in a composite plate may be done using a variety of different theories, such as the classical laminate plate theory, the first order shear deformation theory, the higher order shear deformation theory [8], and so on. In this part, we will construct the link between the classical laminate theory (CLT) and the firstorder shear deformation theory (FSDT) utilising these two theories. and Navier solution is applied to find the desired result. [161]

A. Classical Laminate Plate Theory and Mathematical Modelling of CLT

Any orthotropic continuous fibre laminated composites may be explained by the Classical Lamination Theory (CLT), which is described in this article. The method that was used while developing CLT is quite comparable to the one that was applied when developing load-stress relationships in the fundamental courses on the strength of materials. It is presumed that there was an initial displacement field with loads that were applied. It is possible to characterise a state of stress by using the strain-displacement fields in conjunction with an appropriate constitutive connection. After ensuring that the criteria of static equilibrium are met, one may establish a load-strain relation and, as a consequence of this, one can define a state of stress for each lamina [162].

Fundamental Presuppositions of the Traditional Lamination Theory (CLT)

- The laminate has layers that are each quasihomogeneous and orthotropic in nature.
- When compared to the lateral dimensions, the laminate is rather thin, and it bears load in its plane.
- State of stress is plane stress.
- Any and all displacements are negligible in comparison to the thickness of the laminate.
- There is no break in the displacements anywhere inside the laminate.
- After deformation, straight lines that are normal to the centre surface keep their straightness and maintain their normality to that surface.
- In-plane displacements vary linearly through the thickness,
- Transverse shear strains (γ_{xz} & γ_{yz}) are negligible.
- Transverse normal strain ε_z is negligible compared to the in-plane strains ε_x and ε_y .
- Strain-displacement and stress-strain relations are linear.

The centre of the plate, also known as z = 0, serves as the point of origin for the plate. Assume that u0, v0, and w0 are the displacements in the x, y, and z directions, respectively, at the midplane, and that u, v, and w are the displacements at any point in the x, y, and z directions, respectively. u, v, and w are the displacements at any

point in the x, y, and z directions, respectively. The two displacements in the x-y plane at any point other than the midplane will rely on the axial position of the point as well as the slope of the laminate midplane with respect to the x and y directions [163].



Fig. 3 Bending of the ply

Displacement in x and y-direction

$$u = u_0 - z \frac{\partial w_0}{\partial x} \tag{1}$$

$$v = v_0 - z \frac{\partial w_0}{\partial y} \tag{2}$$

Strain-displacement equation can be written in matrix form as in equation (3).

$$\begin{cases} \varepsilon_{x} \\ \varepsilon_{y} \\ \gamma_{xy} \end{cases} = \begin{cases} \frac{\partial u_{0}}{\partial x} \\ \frac{\partial v_{0}}{\partial y} \\ \frac{\partial u_{0}}{\partial x} + \frac{\partial v_{0}}{\partial y} \end{cases} + z \begin{cases} -\frac{\partial^{2} w_{0}}{\partial x^{2}} \\ -\frac{\partial^{2} w_{0}}{\partial y^{2}} \\ -\frac{\partial^{2} w_{0}}{\partial y^{2}} \\ -\frac{\partial^{2} w_{0}}{\partial x \partial y} \end{cases}$$
(3)
$$\begin{cases} \varepsilon_{x} \\ \varepsilon_{y} \\ \gamma_{xy} \end{cases} = \begin{cases} \varepsilon_{x}^{0} \\ \varepsilon_{y}^{0} \\ \gamma_{xy}^{0} \end{cases} + z \begin{cases} k_{x} \\ k_{y} \\ k_{xy} \end{cases}$$
(4)

The governing equations consist of the behavior of the boundary conditions as well as the behavior of the plate internally. The governing differential equations will be derived by summing the forces and moments on the plate.



Fig. 4 Forces in z-direction

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Fig. 5 Moment on the plate

The governing equation can be obtained by the summing up the forces and moment in x and y-direction

$$\frac{\partial N_x}{\partial x} + \frac{\partial N_{xy}}{\partial y} = 0 \tag{5}$$

$$\frac{\partial N_{xy}}{\partial x} + \frac{\partial N_y}{\partial x} = 0 \tag{6}$$

$$\frac{\partial^2 M_x}{\partial x^2} + \frac{\partial^2 M_{xy}}{\partial x \partial y} + \frac{\partial^2 M_x}{\partial y^2} + q = 0$$
(7)

The equilibrium equation in terms of displacement is as

$$A_{11}\frac{\partial^2 u^o}{\partial x^2} + 2A_{16}\frac{\partial^2 u^o}{\partial x \partial y} + A_{66}\frac{\partial^2 u^o}{\partial y^2} + (A_{12} + A_{66})\frac{\partial^2 v^o}{\partial x \partial y} + A_{26}\frac{\partial^2 u^o}{\partial y^2} - B_{11}\frac{\partial^3 w^o}{\partial x^3} - 3B_{16}\frac{\partial^3 w^o}{\partial x^2 \partial y} - (B_{12} + B_{66})\frac{\partial^3 w^o}{\partial y^2 \partial x} - B_{26}\frac{\partial^3 w^o}{\partial y^3} = 0$$
(8)

$$A_{16}\frac{\partial^2 u^o}{\partial x^2} + (A_{12} + A_{66})\frac{\partial^2 u^o}{\partial x \partial y} + A_{26}\frac{\partial^2 u^o}{\partial y^2} + A_{66}\frac{\partial^2 v^o}{\partial x^2} + 2A_{26}\frac{\partial^2 v^o}{\partial x \partial y} + A_{22}\frac{\partial^2 u^o}{\partial y^2} - B6\frac{\partial^3 w^o}{\partial x^3} - (B_{12} + 2B_{66})\frac{\partial^3 w^o}{\partial x^2 \partial y} - 3B_{26}\frac{\partial^3 w^o}{\partial y^2 \partial x} - B_{26}\frac{\partial^3 w^o}{\partial y^3} = 0$$
(9)

$$D_{11} \frac{\partial^4 w^0}{\partial x^4} + 4D_{16} \frac{\partial^4 w^0}{\partial x^3 \partial y} + 2(D_{12} + 2D_{66}) \frac{\partial^4 w^0}{\partial x^2 \partial y^2} + 4D_{26} \frac{\partial^4 w^0}{\partial x \partial y^3} + D_{22} \frac{\partial^4 w^0}{\partial y^4} - B_{11} \frac{\partial^3 u^0}{\partial x^3} - 3B_{16} \frac{\partial^3 u^0}{\partial x^2 \partial y} - (B_{12} + 2B_{66}) \frac{\partial^3 u^0}{\partial y^2 \partial x} - B_{26} \frac{\partial^3 u^0}{\partial y^3} - B_{16} \frac{\partial^3 v^0}{\partial x^3} - (B_{12} + 2B_{66}) \frac{\partial^3 v^0}{\partial x^2 \partial y} - 3B_{26} \frac{\partial^3 v^0}{\partial y^2 \partial x} - B_{22} \frac{\partial^3 v^0}{\partial y^3} = 0$$
(10)

Navier solution for CLT

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Load and the displacement are considered in the form of furrier transform



Fig. 6 Boundary condition for CLT

Boundary condition

$$w_{0}(x,0) = 0 \qquad w_{0}(x,b) = 0$$

$$w_{0}(0,y) = 0 \qquad w_{0}(a,y) = 0$$

$$M_{yy}(x,0) = 0 \qquad M_{xx}(0,y) = 0$$

$$M_{yy}(x,b) = 0 \qquad M_{xx}(a,y) = 0$$
(11)

$$w_0(x, y) = \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} W_{mn} \sin \frac{m\pi}{a} x \sin \frac{n\pi}{b} y$$
(12)

$$q_0(x,y) = \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} Q_{mn} \sin \frac{m\pi}{a} x \sin \frac{n\pi}{b} y$$
(13)

Where $q_0(x,y)$ represent the load value.

$$Q_{mn} = \frac{4}{ab} \int_0^b \int_0^a q(x, y) \sin \frac{m\pi}{a} x \sin \frac{n\pi}{b} y dx dy$$
(14)

The forces and moments can be expressed as

$$\begin{pmatrix} N_{x} \\ N_{y} \\ N_{xy} \\ M_{x} \\ M_{y} \\ M_{xy} \\ M_{xy} \end{pmatrix} = \begin{bmatrix} A_{11} & A_{12} & A_{13} & B_{11} & B_{12} & B_{13} \\ A_{22} & A_{21} & A_{23} & B_{22} & B_{21} & B_{23} \\ A_{13} & A_{32} & A_{33} & B_{13} & B_{32} & B_{33} \\ B_{11} & B_{12} & B_{13} & D_{11} & D_{12} & D_{13} \\ B_{22} & B_{21} & B_{23} & D_{22} & D_{21} & D_{23} \\ B_{13} & B_{32} & B_{33} & B_{13} & B_{32} & B_{33} \end{bmatrix} \begin{pmatrix} \mathcal{E}_{x}^{0} \\ \mathcal{E}_{y}^{0} \\ \mathcal{K}_{xy} \\ k_{x} \\ k_{y} \\ k_{xy} \end{pmatrix}$$
(15)

B. First-Order Shear Deformation Theory and Mathematical Modelling of FSDT

During the mathematical modeling of the CLT we take many assumptions, now for FSDT some of the assumptions are relaxed to get more practical results.[163]

The only assumption relaxed for FSDT is that the perpendicular to the mid-surface plane is not remained normal after the deformation [164-167].



Fig. 7 Bending of ply in FSDT the normal is not remain normal to mid surface after deformation

Displacements are given by

$$u(x, y, z, t) = u_o(x, y, z, t) + z\varphi_x(x, y, t)$$
(16)

$$v(x, y, z, t) = v_o(x, y, z, t) + z\varphi_y(x, y, t)$$
(17)

$$z(x, y, z, t) = z_o(x, y, z, t)$$
(18)

$$\varphi_x = \frac{\partial u}{\partial z} \qquad \qquad \varphi_y = \frac{\partial v}{\partial z}$$
 (19)

$$\begin{cases} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \gamma_{yz} \\ \gamma_{xy} \\ \gamma_{xy} \end{cases} = \begin{cases} \frac{\partial u_0}{\partial x} + \frac{1}{2} \left(\frac{\partial w_0}{\partial x} \right)^2 \\ \frac{\partial v_0}{\partial y} + \frac{1}{2} \left(\frac{\partial w_0}{\partial y} \right)^2 \\ \frac{\partial w_0}{\partial y} + \varphi_y \\ \frac{\partial w_0}{\partial x} + \varphi_x \\ \frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} + \frac{\partial w_0}{\partial y} \frac{\partial w_0}{\partial x} \end{cases} + z \begin{cases} \frac{\partial \varphi_x}{\partial x} \\ \frac{\partial \varphi_y}{\partial y} \\ \frac{\partial \varphi_y}{\partial y} \\ 0 \\ 0 \\ \frac{\partial \varphi_y}{\partial y} + \frac{\partial \varphi_y}{\partial x} \end{cases} \end{cases}$$

(20)

The equilibrium equation in terms of displacement is as

$$\frac{\partial}{\partial x} \left[A_{11} \frac{\partial u_0}{\partial x} + A_{12} \frac{\partial v_0}{\partial y} + A_{16} \left(\frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} \right) + B_{11} \frac{\partial \varphi_x}{\partial x} \right] \\ + B_{12} \frac{\partial \varphi_y}{\partial y} + B_{16} \left(\frac{\partial \varphi_x}{\partial y} + \frac{\partial \varphi_y}{\partial x} \right) + \frac{\partial}{\partial y} \left[A_{16} \frac{\partial u_0}{\partial x} + A_{26} \frac{\partial v_0}{\partial y} + A_{66} \left(\frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} \right) + B_{16} \frac{\partial \varphi_x}{\partial x} \\ + B_{26} \frac{\partial \varphi_y}{\partial y} + B_{66} \left(\frac{\partial \varphi_x}{\partial y} + \frac{\partial \varphi_y}{\partial x} \right) \right] \\ - \left(\frac{\partial N_{xx}^T}{\partial x} + \frac{\partial N_{xy}^T}{\partial y} \right) = I_0 \frac{\partial^2 u_0}{\partial t^2} + I_1 \frac{\partial^2 \varphi_x}{\partial t^2}$$
(21)

$$\frac{\partial}{\partial x} \left[A_{16} \frac{\partial u_0}{\partial x} + A_{26} \frac{\partial v_0}{\partial y} + A_{66} \left(\frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} \right) + B_{16} \frac{\partial \varphi_x}{\partial x} \right. \\ \left. + B_{26} \frac{\partial \varphi_y}{\partial y} + B_{66} \left(\frac{\partial \varphi_x}{\partial y} + \frac{\partial \varphi_y}{\partial x} \right) \right] + \frac{\partial}{\partial y} \left[A_{12} \frac{\partial u_0}{\partial x} \right. \\ \left. + A_{22} \frac{\partial v_0}{\partial y} + A_{26} \left(\frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} \right) + B_{12} \frac{\partial \varphi_x}{\partial x} \right. \\ \left. + B_{22} \frac{\partial \varphi_y}{\partial y} + B_{26} \left(\frac{\partial \varphi_x}{\partial y} + \frac{\partial \varphi_y}{\partial x} \right) \right] \right]$$

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$$\frac{\partial}{\partial x} \left[KA_{45} \left(\frac{\partial w_0}{\partial y} + \varphi_y \right) + KA_{55} \left(\frac{\partial w_0}{\partial x} + \varphi_x \right) \right] \\ + \frac{\partial}{\partial y} \left[KA_{44} \left(\frac{\partial w_0}{\partial y} + \varphi_y \right) + KA_{45} \left(\frac{\partial w_0}{\partial x} + \varphi_x \right) \right] \\ + \hat{N}_{xx} \frac{\partial^2 w_0}{\partial x^2} + \hat{N}_{yy} \frac{\partial^2 w_0}{\partial y^2} + 2\hat{N}_{xy} \frac{\partial^2 w_0}{\partial x \partial y} + q$$

$$= I_o \frac{\partial^2 w_0}{\partial t^2}$$

$$\frac{\partial}{\partial x} \left[B_{11} \frac{\partial u_0}{\partial x} + B_{12} \frac{\partial v_0}{\partial y} + B_{16} \left(\frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} \right) \right] \\ + D_{11} \frac{\partial \varphi_x}{\partial x} + D_{12} \frac{\partial \varphi_y}{\partial y} + D_{16} \left(\frac{\partial \varphi_x}{\partial y} + \frac{\partial \varphi_y}{\partial x} \right) \\ + \frac{\partial}{\partial y} \left[B_{16} \frac{\partial u_0}{\partial x} + B_{26} \frac{\partial v_0}{\partial y} + B_{66} \left(\frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} \right) \right] \\ + D_{16} \frac{\partial \varphi_x}{\partial x} + D_{26} \frac{\partial \varphi_y}{\partial y} + D_{66} \left(\frac{\partial \varphi_x}{\partial y} + \frac{\partial \varphi_y}{\partial x} \right) \\ - \left[KA_{45} \left(\frac{\partial w_0}{\partial y} + \varphi_y \right) + KA_{55} \left(\frac{\partial w_0}{\partial x} + \varphi_x \right) \right]$$

$$(23)$$

$$\frac{\partial}{\partial x} \left[B_{16} \frac{\partial u_0}{\partial x} + B_{26} \frac{\partial v_0}{\partial y} + B_{66} \left(\frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} \right) \right. \\ \left. + D_{16} \frac{\partial \varphi_x}{\partial x} + D_{26} \frac{\partial \varphi_y}{\partial y} + D_{66} \left(\frac{\partial \varphi_x}{\partial y} + \frac{\partial \varphi_y}{\partial x} \right) \right. \\ \left. + \frac{\partial}{\partial y} \left[B_{12} \frac{\partial u_0}{\partial x} + B_{22} \frac{\partial v_0}{\partial y} + B_{26} \left(\frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} \right) \right. \\ \left. + D_{12} \frac{\partial \varphi_x}{\partial x} + D_{22} \frac{\partial \varphi_y}{\partial y} + D_{26} \left(\frac{\partial \varphi_x}{\partial y} + \frac{\partial \varphi_y}{\partial x} \right) \right. \\ \left. - \left[KA_{44} \left(\frac{\partial w_0}{\partial y} + \varphi_y \right) + KA_{45} \left(\frac{\partial w_0}{\partial x} + \varphi_x \right) \right] \right]$$

$$\left. \left(\frac{\partial M_{xy}^x}{\partial x} + \frac{\partial M_{yy}^y}{\partial y} \right) = I_2 \frac{\partial^2 \varphi_y}{\partial t^2} + I_1 \frac{\partial^2 v_0}{\partial t^2}$$

$$(25)$$

Navier solution for FSDT

Load and the displacement are considered in the form of furrier transform



Fig. 8 Boundary condition for FSDT

Boundary condition

$$w_0(x, 0, t) = 0 \quad w_0(x, b, t) = 0$$

$$w_0(0, y, t) = 0 \quad w_0(a, y, t) = 0$$

$$u_0(x, 0, t) = 0 \quad u_0(x, b, t) = 0$$

$$v_0(0, y, t) = 0 \quad v_0(a, y, t) = 0$$

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$$M_{yy}(x, 0, t) = 0 \quad M_{yy}(x, b, t) = 0$$

$$M_{xx}(0, y, t) = 0 \quad M_{xx}(a, y, t) = 0$$

$$N_{yy}(x, 0, t) = 0 \quad N_{yy}(x, b, t) = 0$$

$$N_{xx}(0, y, t) = 0 \quad N_{xx}(a, y, t) = 0$$

$$\varphi_{x}(x, 0, t) = 0 \quad \varphi_{x}(x, b, t) = 0$$

$$\varphi_{y}(o, y, t) = 0 \quad \varphi_{y}(a, y, t) = 0$$
(26)

$$w_0(x, y, t) = \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} W_{mn}(t) \sin \frac{m\pi}{a} x \sin \frac{n\pi}{b} y \quad (27)$$

$$u_0(x, y, t) = \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} U_{mn}(t) \cos \frac{m\pi}{a} x \sin \frac{n\pi}{b} y \quad (28)$$

$$v_0(x, y, t) = \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} V_{mn} \sin \frac{m\pi}{a} x \cos \frac{n\pi}{b} y$$
(29)

$$\varphi_x(x, y, t) = \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} X_{mn} \cos \frac{m\pi}{a} x \sin \frac{n\pi}{b} y \qquad (30)$$

$$\varphi_{y}(x, y, t) = \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} Y_{mn} \cos \frac{m\pi}{a} x \sin \frac{n\pi}{b} y$$
(31)

$$q(x, y, t) = \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} Q_{mn}(t) \sin \frac{m\pi}{a} x \sin \frac{n\pi}{b} y \qquad (32)$$

The deflection can be expressed as

$$\begin{bmatrix} \hat{S}_{11} & \hat{S}_{12} & 0 & \hat{S}_{14} & \hat{S}_{15} \\ \hat{S}_{12} & \hat{S}_{22} & 0 & \hat{S}_{24} & \hat{S}_{25} \\ 0 & 0 & \hat{S}_{33} & \hat{S}_{34} & \hat{S}_{35} \\ \hat{S}_{14} & \hat{S}_{24} & \hat{S}_{34} & \hat{S}_{44} & \hat{S}_{45} \\ \hat{S}_{15} & \hat{S}_{25} & \hat{S}_{35} & \hat{S}_{45} & \hat{S}_{55} \end{bmatrix} \begin{pmatrix} U_{mn} \\ V_{mn} \\ W_{mn} \\ X_{mn} \\ Y_{mn} \end{pmatrix} = \begin{cases} 0 \\ 0 \\ Q_{mn} \\ 0 \\ 0 \end{cases}$$
(33)

Where stiffness matrix is the function of the A, B and D matrix, Q_{mn} is load coefficient.[165]

3. Result and discussion

The analysis of simply supported composite plate is done with the Matlab programming. The mathematical modeling is done for the plate and Navier solution is found out. The non-dimensional parameters are plotted against each other like the ratio of plate width to thickness plotted against the deflection, modulus ratio also plotted against deflection, the effect of the ply angle and the number of layers also plotted.





In above Fig. 9 as the number of layers are increasing the non-dimensional deflection is decreasing, Also, as the modulus ratio is increasing the non-dimensional deflection is decreasing. If the modulus ratio is less the 20 the non-dimensional deflection is high and considerable variation with respect to the number of layers.





Fig. 10 Non- dimensional deflection variation with respect to modulus ratio for FSDT

From Fig.10 as the number of layers are increasing the deflection is decreasing and also as the modulus ratio is increasing the deflection is also decreasing.



Fig. 11 Non- dimensional deflection variation with respect to ply angle for CLT

From Fig.11 the non-dimensional deflection is decreasing as the ply angle is increasing. As the number of layers is increasing the deflection is affected less or there is less amount of deflection.



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Fig. 12 Non- dimensional deflection variation with respect to width to thickness ratio for antisymmetric angle-ply laminate for CLT and FSDT

From the Fig.12 the variation in the deflection is less when the ratio(a/h) is more the 20. If the ratio is less than 20 the variation of deflection For FSDT is higher than the CLT. When two stacking sequence and the more are taken the variation shown by the CLT is remains constant means not such a considerable change is there but as for the FSDT the change is considerable if the side to thickness ratio (a/h) is below 20.

Conclusions

The above analysis shows that the first order shear deformation theory gives better results than classical laminate theory. The difference is very high for the side to thickness ratio if less than 20. The deflection should increase as the modulus ratio increase. If the number of layers increasing the deflection decreasing. The program made for this is generalized one the output trend will be always same for the inputs.

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